# **Coefficient Of Discharge For A Combined Hydraulic Measuring Device**

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#### Abstract

The aim of this study is to investigate the coefficient of discharge for a combined hydraulic measuring device. For this purpose nine combined models were constructed and manufactured of an aluminum plates of 2 mm thick, the shapes of the models are of rectangular weir with different width (B = 8,10,12cm) is used over a semi-circular gate of a constant diameter (d = 12cm) the distance below the weir edge and the semi-circular gate y is changed three times (y = 5,10,15cm). The analysis of results show that Cd increase as (h/d) increase and for a constant value of (h/d) Cd increase as the width B increase, the values of Cd range from around 0.522 to 0.853 with an average of 0.695. Also Cda decrease as the parameter (y/d), (B/d) and (b/W) increase and at a constant values of that parameters Cda increase as y increase, and the values of Cda range from around 0.61 to 0.74. A multi regression model to estimate Cd for the combined device is estimated with percentage of error  $\pm 10\%$ .

Keywords: combined orifice, combined weirs.

معامل التصريف لأداة مركبة

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## الخلاصة:

يهدف البحث إلى تقدير معامل التصريف Cd لأداة مركبة صنعت لهذه الغاية تسع نماذج من مادة الألمنيوم بسمك 2 ملم .وهذه الأداة المركبة عبارة عن فتحة هدار مستطيلة الشكل بعرض ( B = 8,10,12cm ) وضعت إلى الأعلى من فتحة نصف دائرية بقطر ثابت (d = 12cm) وان المسافة بين الفتحة نصف الدائرية والهدار تتغير شلاث مرات((y = 5,10,15cm)). وقد أشارت نتائج الدراسة بان معامل التصريف Cd يزداد بزيادة (h/d) وترداد قيمة *Cd* بزيادة (y) عند ثبوت (h/d)، وإن قيم معامل التصريف *Cd* تراوحت بين 0.522 إلى 0.853 وإن معدل معامل التصريف (Cda) يقل بزيادة المتغير ات (B/W, B/d, y/d). وتراوحت قيم Cda بين 0.61 إلى 0.74 كما تم استنباط علاقة لتقدير معامل التصريف Cd بدلالة المتغيرات أعلاه بنسبة خطأ تتراوح %10±.

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## **Introduction:**

Weir structures are used in canal, river and reservoir application for flow measurement. Both weirs and gates are used frequently for discharge measurements in open channels. Works concerning the use of sluice gates as discharge measurement structure may be found, e.g by Rajaratham(1977). Weirs and gates may be combined together in one device yielding a simultaneous flow over the weir and below the gate (Negam, 2002). The flow through combined devices may be free when both the flow over weir and below the gate are free or it is termed submerged when the flow below the gate is submerged. The simultaneous discharge can be obtained by adding the overflow discharge to the under flow discharge and making use of interaction factor (Negam, 1996 and Negam et al., 2000). Comprehensive review of the studies dealing the simultaneous flow over weirs and below gates can be found in(Negam et al., 2002) Negam, 2002 propose a general model for predicting the combined flow over weirs and below gates for both free and submerged gates conditions utilizing the basic discharge equations of the weir and gates and an artificial neural networks to predict the simultaneous discharge over weirs and below gates when they have unequal contractions. Then the results of both methods are compared. The aim of the present study is to investigate experimentally the free flow through a combined rectangular weir and below a semi-circular gate.(a combined device) and to propose a model for predicting the discharge coefficient through it.

## **Theoretical analysis:**

Figure 1 shows definition sketch for the free flow over rectangular weir and through submerged semi- circular gate (combined device) of different contraction.



Fig.1. Definition Sketch For the Combined Hydraulic Measuring Device

 $Q_{the}$  is the total discharge through the combined device which is calculated as follows.

$$Q_{the} = Q_w + Q_g \qquad \dots (1)$$

 $Q_g$ : discharge through a semi- circular gate  $(m^3/s)$ .

 $Q_w$ : discharge through a rectangular weir  $(m^3 / s)$ .

 $Q_{the}$ : total theoretical discharge through combined device  $(m^3 / s)$ 

$$Q_{w} = \frac{2}{3}\sqrt{2g}Bh^{\frac{3}{2}} \qquad \dots (2)$$

$$Q_{g} = \frac{\pi\sqrt{2g}}{8}d^{2}H^{\frac{1}{2}} \qquad \dots (3)$$

g :acceleration due to gravity  $(m/s^2)$ .

d: the diameter of the gate (m).

h: the head through the rectangular weir (m).

y: the distance below the weir and over the gate edge (m).

*H*: the total head ;  $H = h + y + \frac{d}{2}(m)$ .

B: the width of the rectangular weir (m).

$$Q_{act} = \operatorname{Cd} Q_{the} \qquad \dots (4)$$

 $Q_{\rm act}$ : the total actual discharge through the standard rectangular weir (l/s)

Cd : the coefficient of discharge for the combined device

For flow through a rectangular weir and below a semi- circular gate (combined device), the discharge  $Q_{act}$  can be expressed by the following functional relation ship:

$$Q_{act} = f(W, d, y, h, B, H, g, \rho, \mu, \sigma) \qquad \dots (5)$$

 $\rho$ : density of water ( $ML^{-3}$ ),  $\mu$ : dynamic viscosity ( $ML^{-1}T^{-1}$ ),  $\sigma$ : surface tension ( $MT^{-2}$ ). Based on eq. 5 and using dimensional analysis (Buckingham  $\pi$  Theorem) shows the following:

$$\frac{Q_{act}}{(g^{\frac{1}{2}}d^{\frac{5}{2}})} = f(\frac{y}{d}, \frac{B}{d}, \frac{h}{d}, \frac{H}{d}, \frac{B}{W}, \frac{\mu}{\rho g^{\frac{1}{2}}d^{\frac{3}{2}}}, \frac{\sigma}{\rho g d^{2}}) \qquad \dots (6)$$

But 
$$\frac{\mu}{\rho g^{\frac{1}{2}} d^{\frac{3}{2}}} = \frac{1}{\text{Re}}$$
, where Re is Reynolds number

and  $\frac{\sigma}{\rho g d^2} = \frac{1}{We}$  and We is Weber number

In terms of  $C_d$  eq. (6) can be written as

$$Cd = f(\frac{y}{d}, \frac{B}{d}, \frac{h}{d}, \frac{H}{d}, \frac{B}{W}, \text{Re}, We) \qquad \dots (7)$$

The effect of Reynolds number and Weber number is assumed to be negligible for the combined device except at a very low head

#### **Experimental set- up:**

The experiments were carried out in horizontal rectangular channel of 10m length, with cross-section 0.3m wide and 0.45m high. The walls of the channel were a toughened

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glass with number of perpex panels incorporated. The bed of the channel consisted of stainless –steel plates. A pair of a justable instrument rails were fitted on the top of the channel sides through the working length of the channel two movable carriages equipped with point gauges were mounted on the rails. Fig. (2) shows the details of the channel used in this study. All series of experiment were conducted in the hydraulic laboratory of water resources engineering department, college of engineering, university of Mosul.



Side View of The Channel

## Fig.2. Channel Lay Out

Nine combined device model were constructed and manufactured of an aluminum plates 2mm thick, these models were of a rectangular weir with different width (B = 8,10,12cm) over a semi-circular gate of constant diameter d = 12cm, the distance below the weir edge y and the semi-circular gate is changed three times (y = 5,10,15cm). The detail of the combined device used is shown in table (1).

Run No.	Width of Weir(B cm)	Distance(Y cm)
1-6	8	5
7-12	8	10
13-17	8	15
18-23	10	5
24-29	10	10
30-34	10	15
35-40	12	5
41-46	12	10
47-51	12	15
	Run No. 1-6 7-12 13-17 18-23 24-29 30-34 35-40 41-46 47-51	Run No.Width of Weir(B cm)1-687-12813-17818-231024-291030-341035-401241-461247-5112

**Table (1): Details of Combined Device Tested** 

#### Hayawi : Coefficient Of Discharge For A Combined Hydraulic Measuring Device...

All combined devices were fixed at distance 1.5m up stream from the channel outlet section and for actual discharge measurement a full width thin-plate sharp-crested rectangular weir of 15cm height were used. The weir was manufactured according to British standards (1965). The head up-stream of the weir and the combined device was measured with a precision point gauges whose least count 0.1mm and the temperature was around  $20^{\circ}C$  during all the experiments.

### **Analysis of Results:**

**a**-Variation of *Cd* with  $\frac{h}{d}$ 

The effect of  $\frac{h}{d}$  on Cd can be studied by conducting tests on a combined device. The discharge coefficient Cd was obtained using equation 4 in which  $Q_{act}$  is the discharge measured by the standard rectangular weir. Figures 3, 4, and 5 shows the relation ship between  $\frac{h}{d}$  and the coefficient of discharge Cd for a width of the rectangular weir opening (B = 8,10,12cm) respectively, it can be concluded that Cd increase as  $\frac{h}{d}$  increase. And for a given constant width B the value of Cd increase as the distance(y) above the gate opening and below the rectangular weir increase.





**b**-Variation of *Cda* with  $\frac{y}{d}$ ,  $\frac{B}{d}$ , and  $\frac{B}{W}$ Figures (6, 7, and 8) shows the relation ship between the average discharge coefficient  $Cda\left[(Cda = \frac{\sum Cd}{n}), n=\text{number of runs for each models}\right]$  and the value of the parameters  $\frac{y}{d}, \frac{B}{d}$  and  $\frac{B}{W}$  respectively. From the Figures it was seen that the average coefficient discharge *Cda* decrease as the values of the parameters,  $\frac{y}{d}, \frac{B}{d}$  and  $\frac{B}{W}$  increase and at a constant values of these parameters *Cda* increase as (y) increase.





**c**- Based on dimensional –analysis multiple regression analysis was applied to the data and  $\frac{h}{d}$ ,  $\frac{y}{d}$ ,  $\frac{B}{d}$ , and  $\frac{B}{W}$  are found to represent the observed coefficient of discharge *Cd* better than any other combination of the dimensionless numbers in equation (7). The best fit is:-

$$Cd = 0.38(\frac{H}{d})^{0.811}(\frac{y}{d})^{-0.163}(\frac{B}{d})^{-0.107}(\frac{B}{W})^{0.376} \dots (8)$$
$$R^{2} = 0.802.$$

Where  $R^2$  is the coefficient of determination

**d**-Variation of experimentally observed values of Cd and values predicted by equation (8) is shown in fig.(9), which shows a good agreement.



The percentage of error between the observed and calculated values of Cd is calculated as follows:

The error distribution in predicting coefficient of discharge (observed Cd) from eq.(4) is shown in fig.(10) the percentage of error is within  $\pm 10\%$  for the total experimental

data .Therefore the combined hydraulic measuring device presented in this study is said to be accurate discharge measuring device .



#### **Conclusions:**

Based on the analysis of experimental study on flow through a combined hydraulic measuring device, the following conclusions are withdrawn:

1. The coefficient of discharge Cd increase as the parameter  $(\frac{h}{d})$  increase, and the values of

Cd range from 0.522 to 0.853 with an average value 0.695.

2. The average coefficient of discharge Cda decrease as the parameters

 $(\frac{y}{d}, \frac{B}{d}, \text{ and } \frac{B}{W})$  increase, with range from 0.61 to 0.74.

3. A multi regression analysis were applied to estimate the coefficient of discharge Cd in relation with the parameters  $(\frac{H}{d}, \frac{y}{d}, \frac{B}{d}, \text{ and } \frac{B}{W})$  with percentage of error ±10% for all data.

### **Notations**:

 $Q_w$ : discharge through a rectangular weir  $(m^3 / s)$ .

 $Q_s$ : discharge through a semi- circular gate  $(m^3 / s)$ .

- g : acceleration due to gravity  $(m/s^2)$ .
- d: the diameter of the gate (m).
- B: the width of the rectangular weir (m).
- h: the head through the rectangular weir (m).
- *y* : the distance below the weir and over the gate edge(m).

*H* : the total head ;  $H = h + y + \frac{d}{2}$  (*m*).

 $Q_{act}$ : the total actual discharge through the standard rectangular weir  $(m^3/s)$ .

 $Q_{the}$ : total theoretical discharge through combined device  $(m^3 / s)$ .

Cda: average of coefficient of the discharge of the combined device

*Cd* : the coefficient of the discharge of the combined device

Re: is Reynolds number We: is Weber number W: channel width (m).

- $\rho$ : density of water  $(kg/m^3)$
- $\mu$ : dynamic viscosity (*Pa.s*)
- $\sigma$ : surface tension (N/m)

## **References:**

- 1. British Standard Institution, 1965".Methods of Measurements of Liquid Flow in Open Channels". London, England.
- 2. French, R.H(1986)."Open-Channel Flow Hydraulics,pp.343-353.Cgraw Hill Book Co., Newyurk.
- Negm, A.M.1996."Discharge Prediction Model for Simultaneous Underflow Overflow". Proc.6<sup>th</sup> .Int.Symp. on Flow Modeling and Turbulence Measurements. Florida, USA, sep.8-10.
- Negm, A.M,Abedel-Al.G.M.and Ei-saiad A.A.2000. "Generalized Discharge Equation for Proposed Simultaneous Flow Structures". Proc.Al-A zhar Eng 6<sup>th</sup> Inst.conf.(AEIC). Cairo Egypt. sep. 1-4.4:247-257.
- 5. Negm, A.M.2002a" Prediction of Hydraulic Design Parameters of Expanding Stilling Basins Using Neural Networks". Egyption Journal of Engineering Science and Technology(EJEST).6(1):1-24.
- Negm, A.M.2002b."Analysis and Modeling of Simultaneous Flow Through Box Culverts and Over Contracted Broad-Crested Weirs". proc.of 5<sup>th</sup> Ins.Conf.on Hydro-Science and Engineering. Warsaw. Poland(CR-Room and Booklet of Abstracts).
- 7. Negm, A.M., Al-Brahim A.M. and Alhamid A.A.2002. "Combined Flow Over Weirs and Under Gates". Journal of Hydraulic Research, 40(4).
- Negm, A.M.2002C. "Discution of Analysis and Formation of Flow Through Combined V-notch Gate Device". By Alhamid A.A., Journal of Hydraulic Research 40(4).
- 9. Rajaratnam, N.(1977), "Free Flow Immediately Below Sluice Gates, proc. ASCE, J. Hydraulics Div., Vol.103 No.Hy4, pp.345-351.
- Subramanya, k.(1986) "Flow in Open Channels" ,pp.219-226, Tata Mc-Graw-Hill Publishing Company Ltd., New Delhi, First Revised Edition, 7<sup>th</sup> reprint.

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